

this section is more obvious for larger μ values, which can be illustrated in the following section.

Mass Ratio

Figure 2 shows the bridge-corresponded natural frequency ω_b of the VBI system with respect to μ for $\beta=0.01, 0.5, 0.99, 1.01, 1.209, 1.458,$ and 4 . In this figure, ω_b is approximately equal to ω_{b0} when μ approaches 0, illustrating that vehicle with relatively small mass may hardly affect the natural frequency of the interaction system. For the case with $\beta < 1$, ω_b monotonously increases while μ increases; for the case with $\beta > 1$, ω_b monotonously decreases while μ increases. Such an observation clearly indicates that larger deviation of ω_b from ω_{b0} is induced by larger vehicle-to-bridge mass ratio, i.e. larger vehicle mass or smaller bridge mass.

Figure 3 shows the variation of natural frequency of the VBI system with respective to μ under resonance condition, i.e. the natural frequency ω_v of the vehicle alone is equivalent to the natural frequency ω_b of the bridge alone, namely $\beta=1$. It is observed that the natural frequencies of the two modes deviate symmetrically from the original one and deviations increase as μ increases. Compared with non-resonance conditions, the deviations of the natural frequency of the VBI system are larger for the same μ value.

Conclusions

Observed from the analytical formula shown in Eq.(1), the variability in the natural frequency of a VBI system is dominated by both the frequency ratio and mass ratio of the vehicle to the bridge. For the frequency ratio, the deviation of bridge-related natural frequency of VBI system from the natural frequency of the bridge alone reaches its maximum at near resonance conditions. For the mass ratio, the deviations increase as the mass ratio increases. Also, it is illustrated that a vehicle with relatively small mass or natural frequency may hardly affect the natural frequency of the interaction system.

The analytical results as well as the present figures can be useful in bridge seismic designs, especially for the cases that large frequency variability may occur. More numerical studies should be performed with more complicated VBI models and more field experiments should be carried out to verify the applicability of this analytical formula to the real bridge structures, which will be the next step of this study.

Reference

- 1) K.C. Chang and C.W. Kim, Variability in Bridge frequency induced by a parked vehicle, *Proc.of the 24th KKCNN Symposium on Civil Engineering, Hyogo, Japan*, pp.75-78, 2011.

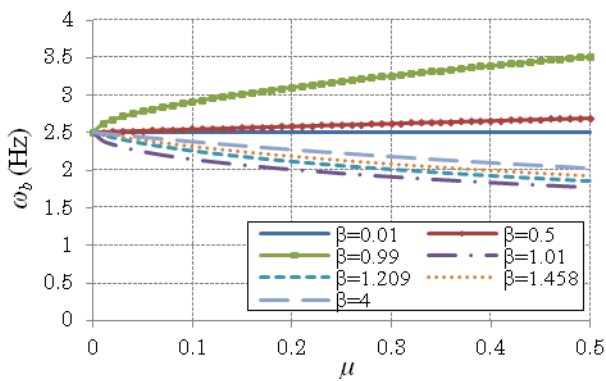


Fig. 2. Bridge-related natural frequency ω_b of the VBI system v.s. mass ratio μ .

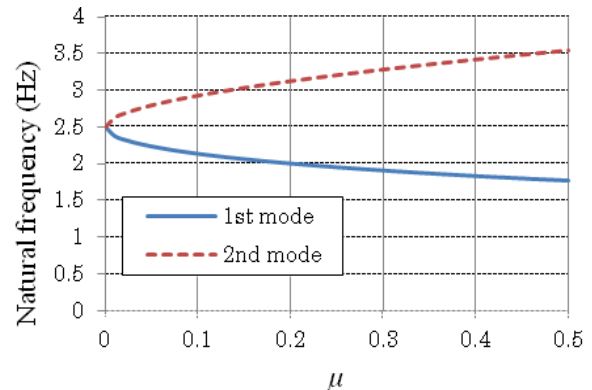


Fig.3. Natural frequency of the VBI system v.s. mass ratio μ in resonance case.