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# EXTENDED MONTE CARLO METHOD IN STRUCTURAL RELIABILITY

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## INTRODUCTION

In the reliability theory of structural design, probability of structural failure  $P_T$  may be very small with order  $10^{-8}$  to  $10^{-6}$  in reality. This probability of failure  $P_T$  is defined as

$$P_f = P(S > R) = 10^{-3}$$
 to  $10^{-6}$  order ......(1)

where  $P(\ )$  reads "probability that". R is the overall resistance of structure and S is the load applied upon structure.

In the case of normal distributions of both S and R, the theoretical solution of  $P_f$  may be evaluated as<sup>1)</sup>

$$P_f = 1 - \Phi \left\{ \frac{\mu_R - \mu_S}{\sqrt{\sigma_R^2 + \sigma_S^2}} \right\} \quad \dots (2)$$

where  $\mu_R$  and  $\mu_S$  are respectively the means of R and S.  $\sigma_R$  and  $\sigma_S$  are the standard deviations of R and S respectively.  $\Phi(\ )$  is the probability distribution function of normalized Gaussian variables.

In general, R is given as a function of many structural elements which may be random variables. Therefore, the distribution of R is not necessarily normal nor a well known pattern of a probability model. The similar argument may ascertain that S may not be described with typical distribution laws.

For example, consider the reliability of an axially loaded reinforced concrete member. The ultimate strength of the column is given by

$$R = k f_c' A_c + f_{sy} A_s$$
 (ACI code)

If  $kf_{o'}$  (concrete strength),  $f_{sy}$  (yield strength of steel) and  $A_c$  (concrete area) are considered to be random variables, the ultimate strength (or we may call it the resistance R) also become random variables. In this case, even the probability distribution functions of  $kf_{o'}$ ,  $f_{sy}$  and  $A_c$  are given by well known distributions such as normal

distribution functions, it is very difficult to obtain the theoretical distribution of R and consequently the evaluation of eq.(1) is very hard to be attained.

Another example is given by the analysis of a tainter gate problem (4). The resistance R of the gate is given by the following complicated function of random variables;

$$R = \frac{\pi^2 EI}{l^2} \left\{ \frac{(\alpha_1{}^s + \alpha_2{}^s + \alpha_3{}^s + (\alpha_1{}^d + \alpha_2{}^d + \alpha_3{}^d)}{(\alpha_1{}^s + \alpha_1{}^d)^{1.5} + \sqrt{\frac{I_1}{I_2}}(\alpha_2{}^s + \alpha_2{}^d)^{1.5} + \sqrt{\frac{I_1}{I_3}}(\alpha^3{}_3 + \alpha^d{}_3)^{1.5}} \right\}^2$$

where  $\alpha_i^s$ ,  $\alpha_i^a$  (i=1,2,3) are random variables. Again the evaluation of the probability distribution of R is not feasible.

In a case where the distribution pattern of overall resistance R is numerically given as an experimental study, these values are stored in computer for the use of the extended Monte Carlo method described in the next section.

In any one of the above cases, it is very difficult if not impossible to obtain the theoretical solution of  $P_f$  as in eq(2). Monte Carlo approach then becomes a very powerful means to evaluate the numerical value of  $P_f$ . However, it is noted that for the order  $10^{-3}$  to  $10^{-6}$  of  $P_f$ , at least more than  $10^3$  to  $10^6$  repeated trials are required and the computer time becomes a criteria to employ a Monte Carlo method.

In this paper, an extended Monte Carlo method is developed to evaluate  $P_f$  of order  $10^{-8}$  to  $10^{-6}$  with far less than  $10^3$  to  $10^6$  repeated trials. A simple conditional probability law is the basic concept in this approach.

## EXTENDED MONTE CARLO METHOD

Equation (1) can be described as a Venn diagram shown in Fig. 1. The hatched area is the event of structural failure. A Monte Carlo method based upon eq. (1) is for the ith trial, first to generate random variables  $a_i$  and  $b_i$  independ-

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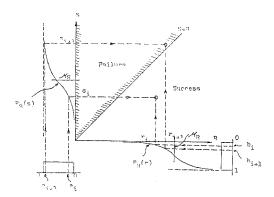


Fig. 1  $P_f = P(S > R)$ 

ently from a uniform probability distribution with the range of zero to one as shown in Fig. 1. Then sample realizations  $s_i$  and  $r_i$  may be obtained from the relations  $s_i = F_S^{-1}(a_i)$  and  $r_i = F_R^{-1}(b_i)$  respectively.  $F_S(s)$  and  $F_R(r)$  are the probability distribution functions of S and R respectively. If  $s_i < r_i$ , the structure is safe and the next trial is to be proceeded. Whenever  $s_i > r_i$ , keep the tally of the structural failure. After sufficient trials, the probability of failure  $P_f$  in eq.(1) can be approximated by the following estimator

$$\hat{P}_f = n/N \cdots (3)$$

where n is the number of failure and N is the total number of independent trials. As can be seen in the above description, the method is very simple except for the many repetitions of trials which are not realistic because of the computer time.

To avoid this technical difficulty, this paper developes an extension of the Monte Carlo method in the following paragraphs.

Consider now the probability of an event of  $(S>R\cap S>S_0\cap R< R_0)$  where  $S_0$  and  $R_0$  are deterministic such that  $S_0< R_0$ .

Following a conditional probability law, we have

$$P(S>R \cap S>S_0 \cap R < R_0) = P(S>R)P(S>S_0 \cap R < R_0/S>R) \cdots (4)$$
 or 
$$P(S>R \cap S>S_0 \cap R < R_0) = P(S>S_0 \cap R < R_0) = P(S>S_0 \cap R < R_0)P(S>R/S>S_0 \cap R < R_0) \cdots (5)$$

Consequently from eqs. (4) and (5)

$$P_f = P(S > R) = \frac{P(S > S_0 \cap R < R_0)P(S > R/S > S_0 \cap R < R_0)}{P(S > S_0 \cap R < R_0/S > R)} = \frac{P(S > R_0 \cap R < R_0)P(S > R_0 \cap R < R_0)}{P(S > R_0 \cap R < R_0)}$$

For the improvement (shortening) of computer time, we will consider the use of the right hand side of eq. (6) to evaluate the probability of failure  $P_{\mathcal{I}}$ .

Let

$$P_1 = P(S > S_0 \cap R < R_0) \cdots (7)$$

$$P_{1/f} = P(S > S_0 \cap R < R_0/S > R) \cdots (8)$$

$$P_{f/1}=P(S>R/S>S_0\cap R< R_0) \cdots (9)$$

Then eq.(6) can be expressed as

$$P_f = P_1 P_{f/1} / P_{1/f}$$
 .....(10)

 $P_1$  is the probability that  $S>S_0$  and  $R< R_0$  simaltaneously. If the events  $(S>S_0)$  and  $(R< R_0)$  are independent, then

$$P_1 = P_{11}P_{12} = P(S > S_0)P(R < R_0) \cdots (11)$$

In the Monte Carlo evaluation of  $P_1$ , it is obvious that the number of trials can be greatly reduced if appropriate values  $S_0$  and  $R_0$  are assigned.

 $P_{1/f}$  is the probability that sample realization S is greater than  $S_0$  and at the same time R is less than  $R_0$  under the condition of S > R When the failure occurs, it may be highly expected that the load is great  $(S \gg S_0)$  and the resistance R is small  $(R \ll R_0)$ . Therefore, in the Monte Carlo simulation, the number of trials can be also reduced for the stable estimation of  $P_{1/f}$ .

On the contrary to  $P_{1/f}$ ,  $P_{f/1}$  is the probability that the structure fails under the condition of of  $S > S_0$  and  $R < R_0$ . The number of trials is also expected to be small.

Consequently, once  $P_1$ ,  $P_{f/1}$ , and  $P_{1/f}$  are evaluated with reasonable number of trials, the probability of failure  $P_f$  can be determined from eq.(10). In other words, with much less than  $10^3$  to  $10^6$  trials, the probability of failure of the order  $10^{-3}$  to  $10^{-6}$  can be estimated.

It is noted that if  $S_0$  is chosen to be very small and  $R_0$  very large, eq.(10) becomes trivial, since in this case

$$P_1 = P(S > 0 \cap R < \infty) = 1.0$$
,  
 $P_{1/f} = P(S > 0 \cap R < \infty/S > R) = 1.0$ 

and

$$P_{f/1} = P(S > R/S > 0 \cap R < \infty) = P(S > R)$$
.

Thus, both sides of eq.(10) is of identical form; P(S>R)=P(S>R).

Thus, the reduction of trials can not be retained. If on the other hand,  $S_0$  and  $R_0$  are chosen such that  $S_0 > R_0$ , the condition  $S > S_0 \cap R < R_0$  automatically means S > R. Therefore we have

$$P(S>R/S>S_0 \cap R< R_0)=1.0$$

and

$$P(S > S_0 \cap R < R_0/S > R)$$
  
=  $P(S > S_0 \cap R < R_0)/P(S > R)$ 

since

$$(S>S_0\cap R< R_0)\subset (S>R)$$
.

Thus, eq. (10) reduces again that P(S>R)=P(S>R) which is trivial. Therefore, the optimum  $S_0$  and  $R_0$  may be determined through the parametric analysis.

For the evaluation of  $P_1$  in eq. (11), the sample realization  $s_i$  is to be filtered out through  $F_S(s)$  into which random number  $a_i$  in the range of zero to one is supplied. Repeating the trials and keeping the tally whenever we have  $s_i > S_0$ ,  $P_{11} = P(S > S_0)$  can be obtained by an estimator;

$$\hat{P}_{11} = n_1/N_1$$
 .....(12)

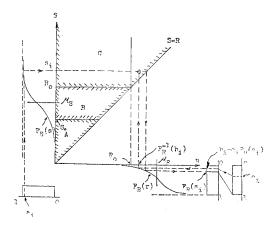
where  $n_1$  is the number of the event  $(s_i > S_0)$  occurring in  $N_1$  independent trials. Similarly  $P_{12} = P(R < R_0)$  can be obtained by

$$\hat{P}_{12} = n_2/N_2$$
 .....(13)

where  $n_2$  is the number of the event  $(r_1 < R_0)$  in  $N_2$  trials.

In what follows, the evaluation of  $P_{1/f}$  and  $P_{f/1}$  is discussed.

Consider the Monte Carlo evaluation of  $P_1/f$ . The event  $(S>S_0 \cap R < R_0/S>R)$  is illustrated in Fig. 2. First generate  $a_i$  from a uniform dis-



**Fig. 2**  $P_{1/f} = P(S > S_0 \cap R < R_0/S > R)$ 

tribution between zero to one. The sample realization  $s_i$  can be obtained from  $s_i = F_S^{-1}(a_i)$ . Since the condition S > R is required in this case,  $b_i$  is to be generated from a uniform distribution between zero to  $F_R(s_i)$ . This means first to generate  $c_i$  from a uniform distribution from zero to one and then to obtain  $b_i$  such that  $b_i = c_i F_R(s_i)$ . The sample realization  $r_i$  is now filtered out from

 $r_i = F_R^{-1}(b_i)$  so that the condition  $s_i > r_i$  is satisfied. For the sample set  $(s_i, r_i)$ , examine if the event  $(s_i > S_0 \cap r_i < R_0)$  occurs. If it does, keep the tally and go to the next trial. After many repetition one may have the probability of event  $(S > S_0 \cap R < R_0/S > R)$  by the following estimator.

$$P_{1/f} = n_3/N_3 \cdots (14)$$

where  $n_3$  is the number of the event occurring in  $N_3$  trials.

It is noted that the range of distribution of  $b_i$  is variable since the upper bound  $F_R(s_i)$  is dependent upon the sample realization  $s_i$ . It is also observed that if  $s_i$  is less than  $S_0$ , the event A indicated in Fig. 2 occurs against any realization of  $r_i$ . Thus the interesting event can not be possible. If  $S_0 < s_i < R_0$ , the event B occurs against any  $r_i$ . Thus the interesting event always occurs. Therefore it is not necessary to generate  $r_i$  in these cases except for the recording the event in the tally.

Next, let us discuss of  $P_{\mathcal{I}/1}$ . The event  $(S > R/S > S_0 \cap R < R_0)$  is shown in Fig. 3. Since the

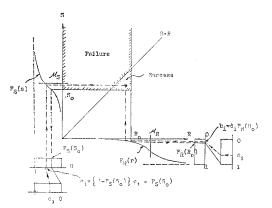


Fig. 3  $P_{f/1} = P(S > R/S > S_0 \cap R < R_0)$ 

conditions  $S > S_0$  and  $R < R_0$  are required,  $a_i$  is to be generated from a uniform distribution between  $F_S(S_0)$  to unity, whereas  $b_i$  is generated from a uniform distribution between zero to  $F_R(R_0)$ . Next generate  $s_i$  and  $r_i$  such that  $s_i = F_S^{-1}(a_i)$  and  $r_i = F_R^{-1}(b_i)$ . Keep the tally whenever the sample realization of  $s_i$  and  $r_i$  satisfies  $s_i > r_i$ . After sufficient trials, the conditional probability of failure  $P_{f/1}$  can be estimated in the similar manner;

$$\hat{P}_{f/1} = n_4/N_4 \cdots (15)$$

where  $n_4$  is the number of the event  $(S>R/S>S_0\cap R< R_0)$  in  $N_4$  trials. As described above, eq. (10) attains a reasonable Monte Carlo evalua-

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tion of the probability of structural failure.

Suppose the external load S is considered to be deterministic as S=a and only the the resistance R is probabilistic. Then, the extended Monte Carlo method can be reduced to the following way;

$$P(a>R\cap R< R_0) = P(a>R)P(R< R_0/a>R)$$
 .....(16)

or

$$P(a>R\cap R< R_0) = P(R< R_0)P(a>R/R< R_0)$$
 .....(17)

From eqs. (16) and (17),

$$P_{f} = P(a > R) = \frac{P(R < R_{0})P(a > R/R < R_{0})}{P(R < R_{0}/a > R)}$$
.....(18)

If  $R_0$  is chosen such that  $a < R_0$ , then

$$R < R_0 \supset a > R$$

Hence

$$P(R < R_0/a > R) = 1.0$$

Therefore eq. (18) becomes

$$P_f = P(a > R) = P(R < R_0)P(a > R/R < R_0)$$
....(19)

It is clear that use of the right hand side of eq. (19) reduces the number of trials in the Monte Carlo approach.

## ERROR STATISTICS

For the evaluation of true probability of failure  $P_f$ , estimators  $\hat{P}_{11}$ ,  $\hat{P}_{12}$ ,  $\hat{P}_{f/1}$  and  $\hat{P}_{1/f}$  are employed in the extended Monte Carlo method. In other words, the true probability of failure from eqs. (10) and (11)

$$P_f = P_{11}P_{12}P_{f/1}/P_{1/f}$$
.....(20)

are estimated either by the direct approach

$$\hat{P}_f = n/N$$
 .....(21)

or by the extended Monte Carlo approach

$$\hat{P}_{f'} = \hat{P}_{11} \hat{P}_{12} \hat{P}_{f/1} / \hat{P}_{1/f}$$

$$= (n_1/N_1)(n_2/N_2)(n_3/N_3)/(n_4/N_4) \cdots (22)$$

Although it was discussed that use of eq. (22) can save considerable amount of computer time, the accuracy of  $\hat{P}_{f}$  of eq. (22) must be examined in the comparison with the accuracy of  $P_{f}$  of eq. (21).

The estimators  $\hat{P}_f$ ,  $\hat{P}_{11}$ ,  $\hat{P}_{12}$ , ... are all random variables since they are given as the fraction of number of interesting events occurring in the independent trials and in fact they are binomially distributed<sup>2)</sup> as, for example;

$$P(\hat{P}_{f} = n/N \leq b) = \sum_{n=0}^{Nb} {N \choose n} P_{f}^{n} (1 - P_{f})^{N-n} \dots (23)$$

with the mean

and the variance

$$\sigma^2 n/N = P_f(1-P_f)/N \cdots (25)$$

## (1) Confidence Interval of $P_f$ based on eq. (21)

Despite of the binomial distribution of  $\hat{P}_f = n/N$ ,  $\hat{P}_f$  can be assumed to be approximately normally distributed due to the central limit theorem, when N is large number although  $P_f$  is very small value.

Thus, we put

$$\hat{P}_f = N\left(P_f, \sqrt{\frac{P_f(1 - P_f)}{N}}\right) \cdots (26)$$

where N(a, b) means normal distribution with mean a and standard deviation b. Therefore  $(1-\alpha)$  % confidence interval can be obtained as follows.

$$P\left(\left|\frac{\hat{P}_f - P_f}{\sqrt{\frac{P_f(1 - P_f)}{N}}}\right| \le K_{\alpha/2}\right) = 1 - \alpha$$

Solving this for  $P_f$  and after some approximation.

$$P\left(\hat{P}_{f} - K_{\alpha/2}\sqrt{\frac{\hat{P}_{f}}{N}} \leq P_{f} \leq \hat{P}_{f} + K_{\alpha/2}\sqrt{\frac{\hat{P}_{f}}{N}}\right)$$

$$= 1 - \alpha \qquad (27)$$

Or the confidence interval is

$$|P_f - P_f| \leq K_{\alpha/2} \sqrt{\frac{P_f}{N}} \cdots (28)$$

For example, 95% confidence interval is given by

$$|P_f - \hat{P}_f| \leq 1.96 \sqrt{\frac{\hat{P}_f}{N}} \cdots (29)$$

(2) Confidence Interval of  $P_f$  based on eq.(22) Estimators  $\hat{P}_{11}$ ,  $\hat{P}_{12}$ , ... are similarly assumed to be normally distributed. For example,

$$\hat{P}_{11} = N\left(P_{11}, \sqrt{\frac{P_{11}(1-P_{11})}{N_1}}\right)$$
....(30)

Expand into Taylor series<sup>3)</sup> the multivariate function of  $\hat{P}_{f'}$  given by eq. (22). Then we can approximate

$$E(\hat{P}_{f'}) \cong \frac{P_{11}P_{12}P_{f/1}}{P_{1/f}} = P_f \cdots (31)$$

For Example $P_f = 10^{-4}$ , $P_{11} = P_{12} = P_{f/1} = P_{1/f} = 10^{-2}$											
_	N	95% Confidence Interval of $ P_f - \hat{P}_f $	Equivalent to N	95% Confidence Interval of $ P_f - \hat{P}_{f'} $							
	104	1.96*10-4	N <sub>i</sub> =10 <sup>2</sup>	3.96*10-4							
	105	0.63*10-4	$N_i = 10^3$	1.22*10-4							
	106	0.20*10-4	$N_i = 10^4$	0.39*10-4							
	107	0.06*10-4	$N_i = 10^5$	0.12*10-4							
	108	0.02*10-4	$N_i = 10^6$	0.04*10-4							

Table 1 Comparison of 95% Confidence Interval

i=1,2,3,4

$$\cong \left\{ \frac{P_{11}P_{12}P_{f/1}}{P_{1/f}} \right\}^{2} \left\{ 1/P_{11}N_{1} \right\} + 1/(P_{12}N_{2}) + 1/(P_{f/1}N_{3}) + 1/(P_{1/f}N_{4}) \right\} \cdots (33)$$

Thus,

$$\sigma^2 \hat{P}_f \cong P_f^2 \beta^2$$
 ....(34)

where

$$\beta^{2} = 1/(P_{11}N_{1}) + 1/(P_{12}N_{2}) + 1/(P_{f/1}N_{3}) + 1/(P_{1/f}N_{4}) \quad \dots (35)$$

Since  $\hat{P}_{J'}$  is approximated as a linear function of  $\hat{P}_{11}$ ,  $\hat{P}_{12}$  etc. by the Taylor series and since  $\hat{P}_{11}$ ,  $\hat{P}_{12}$  etc. can be nearly normally distributed,  $\hat{P}_{J'}$  is also normally distributed. Thus,

$$\hat{P}_f' \cong N(P_f, P_f \beta) \cdots (36)$$

The  $(1-\alpha)\%$  confidence interval is now obtained as follows.

$$P\left(\left|\frac{\hat{P}_{f'}-P_{f}}{P_{f}\beta}\right| \leq K_{\alpha/2}\right) = 1 - \alpha \cdots (37)$$

Therefore,

$$|\hat{P}_f' - P_f| \leq K_{\alpha/2} \beta P_f$$

For example, the 95% confidence interval is

$$|\hat{P}_f' - P_f| \leq 1.96\beta P_f \quad \dots \tag{38}$$

It is noted that since  $P_f$  and  $\beta$  are unknown, the evaluation of eq.(38) is impossible in the strict sense. However, for the purpose of comparison of the accuracy of  $\hat{P}_f$  and  $\hat{P}_{f'}$ , the right hand side of eq.(38) is replaced by

$$1.96\beta P_{t} \approx 1.96\hat{P}_{t}$$

$$\times \sqrt{\frac{1}{\hat{P}_{11}N_1} + \frac{1}{\hat{P}_{12}N_2} + \frac{1}{\hat{P}_{f/1}N_3} + \frac{1}{\hat{P}_{1/f}N_4}} \cdots (39)$$

Based on eqs. (29) and (39), the comparison are made in Table 1.

Table 1 shows the confidence intervals of estima-

tion of the true probability of failure by both the direct approach and extended Monte Carlo approach. For example, the probability of failure  $P_f$  is supposed to be estimated as  $\hat{P}_f = 10^{-4}$  by the direct approach eq.(21) after  $N=10^7$  trials. On the other hand,  $N_1=N_2=N_3=N_4=10^5$  trials are supposed to be needed to estimate  $P_{11}=P_{12}=P_{f/1}=$  $P_{1/f}=10^{-2}$  respectively. Then from the table 1, it can be seen that the first approach gives 0.06× 10-4 of the 95% confidence interval of  $|P_f - \hat{P}_f|$ , whereas the second approach gives  $0.12 \times 10^{-4}$ although this approach needs all together 4×105 trials at most. If the computer program is effectively coded to include some of the four trials in a single loop, the number of trials becomes much less than 4×105. Thus, it is clear that use of  $P_{f'}$  sacrifies the degree of prediction accuracy of  $P_f$ . However, it is noted that at least the order estimation of  $P_f$  can be attained only by  $P_{f'}$  unless otherwise the lengthy computer time is not permitted.

## NUMERICAL EXAMPLE

For the demonstration purpose, both structural resistance R and load S are assumed to be normally distributed with the reason that prior to the Monte Carlo evaluation, the true probability of structural failure  $P_f$  can be obtained by eq. (2). Therefore, the accuracy of  $\hat{P}_f$  can be examined. It is noted, however, that even for any distribution of R and S, the extended Monte Carlo method can be applied.

Assume R=N(10.0, 1.0) and S=N(6.0, 1.0). The first and second arguments are mean and standard deviation respectively. The dimensions are not specified herein because of the demonstration. From eq. (2), then the true  $P_f$  is calculated as  $P_f=0.233*10^{-2}$ .

For several combinations of  $S_0$  and  $R_0$ ,  $P_f$  values were estimated by eq. (22). For a few sets of  $S_0$  and  $R_0$  values, the results are summarized in Table 2 together with the confidence interval.

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$S_0$	$R_0$	$N_i$	$\hat{P}_{11}$	$\hat{P}_{12}$	$\hat{P}_{1/f}$	$\hat{P}_{f/1}$	$\hat{P}_f{}'$	95% Confidence Interval
6.0	8.0	1000	0.539	0.280*10-1	0.519	0.781*10~1	0.227*10-2	0.104*10-2
		2000	0.549	0.285*10-1	0.518	0.815*10-1	0.245*10-2	0.073*10-2
6.0	9.0	1000	0.539	0.186	0.531	0.290*10-1	0.548*10-2	0.095*10-2
		2000	0.549	0.187	0.532	0.293*10-1	0.562*10-2	0.067*10-2
6.0	10.0	1000	0.539	0.454	0.531	0.501*10-2	0.231*10-2	0.207*10-2
		2000	0.549	0.454	0.532	0.501*10-2	0.234*10-2	0.146*10-2
6.0	11.0	1000	0.539	0.785	0.531	0.200*10-2	0.159*10-2	0.325*10-2
		2000	0.549	0.783	0.532	0.250*10-2	0.202*10-2	0.206*10-2
6.0	12.0	1000	0.539	0.965	0.531	0.400*10-2	0.392*10-2	0.231*10-2
		2000	0.549	0.962	0.532	0.250*10-2	0.248*10-2	0.205*10-2
7.0	10.0	1000	0.212	0.454	0.224	0.140*10-1	0.602*10-2	0.131*10-2
		2000	0.217	0.454	0.227	0.175*10~1	0.758*10-2	0.084*10-2
7.0	11.0	1000	0.212	0.785	0.224	0.501*10-2	0.372*10-2	0.209*10-2
		2000	0.217	0.783	0.227	0.816*10-2	0.611*10-2	0.118*10-2

Table 2 Results of Numerical Example

True Probability  $P_f = 0.233*10^{-2}$ 

For example, choose  $S_0$ =6.0 and  $R_0$ =10.0. If  $N_1$ = $N_2$ = $N_3$ = $N_4$ =1000 trials are made, the estimator  $\hat{P}_J$ '=0.231×10<sup>-2</sup> can be obtained with 0.207×10<sup>-2</sup> of 95% confidence interval.

In the case of  $N_1 = N_2 = N_3 = N_4 = 2000$  trials,  $P_f' = 0.234 \times 10^{-2}$  with  $0.146 \times 10^{-2}$  of 95% confidence interval

Since only a small computer was available at hand, optimum values of  $S_0$  and  $R_0$  were not discussed herein. However, it is obvious that if  $S_0$  and  $R_0$  are close to their mean values with the condition  $S_0 < R_0$ , the reduction of computer time can be expected.

It is observed that the order estimation is at least successfully obtained within less repititions of trials than by the direct approach. Note that the confidence interval in Table 2 is purely of statistical nature and does not consider the error of computer calculation.

## CONCLUSIONS

An extended Monte Carlo method is developed by applying a simple conditional probability theory for the evaluation of very low probability of structural failure. Numerical examples are given for the purpose of demonstration.

As the summary, the following conclusions are made;

- An extended Monte Carlo method makes it feasible to technically obtain a very low probability of structural failure within reasonable computer time.
- (2) The accuracy of prediction of true probability of failure by this approach is discussed and an order estimation of  $P_f$  is assured through the discussion of error statistics.

Appreciation is forwarded to Mr. Ishii, graduate student of civil engineering, Musashi Institute of Technology for his help in the computer calculations.

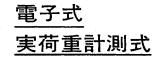
### REFERENCES

- Hoshiya, M: Probabilistic Structural Analysis in Japanese, Kajima Shuppan Kai, March 1973, pp. 117
- Parzen, E.: Modern Probability Theory and its Applications, McGraw Hill, pp. 53
- Benjamin, J. R., Cornell, C. A.: Probability, Statistics and Decision for Civil Engineers, McGraw Hill, New York, 1970
- M. Hoshiya and S. T. Spence, Reliability Analysis of a Tainter Gate, Proc. of JSCE, No. 183. Nov. 1970

(Received May 21, 1973)

## 電気・油圧サーボシステム・自記計測のマルイ団団 創業50年

# 新しい万能材料試験機





『要素を省きました。

負荷荷重の検出は特

電子式万能試験機

MIE-734型

※ ※ ※ ※ ※ 的 温度変化除く特殊電気回 荷 特殊ロードセルは引張強度 測指示は自動平衡装置利用 重負荷は多連式ポンプにて行う

計測 直 接計測して、 構と負荷機構の分離

従来の間接的計測の不可抗 殊 口 F セ ル 力

# **実荷重計測** 生みだします。 くの利点を

## 油 9 7 8 応答性早く ⑥負荷中レ ⑤再現性いちじるし ④ 感度上昇 圧系統 自記自動化が容易になっ 破断ショック影響受けな 」調容易になった ンジ切換えできる 05秒以内

\* 詳細ご一報下さい。 すぐ参上します。

電子式?

駆使して

使しています

指特殊の

動きはタコゼネレーターによる自動平ードセルは、D·T·Fを利用精度01 μ このように計測はすべて電子回路を

完全な電子式機構

特殊ロードセル方式のための

計測 徝 は関係 は負荷す あ りません るだけ

自記自動化のトップをめざす

\*\*\*\*\*\* 圖井製作所

멂 目

質試験 機 非破壞試験機 土 アスファルト試験機 温調試験機 コンクリート試験機 水理試験装置 セメント試験機 材料試験機



た

株式会社

③操作簡単 ②故障発生減 ①正確な計測

少

信頼を旨とす

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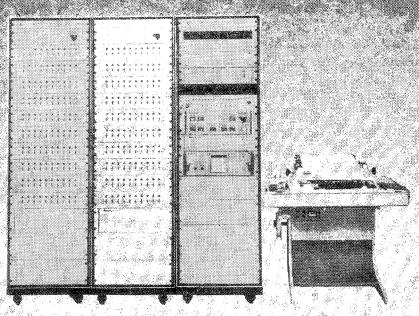
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2





ンでミニコンにインプットして解 析処理し、その出力を入出力タイ プライタにて印字作表する装置で

- ●周辺機器は豊富なため最適のシ ステムが選択できる
- ●中央の大型コンピュータと連結

起電力の影響がない

●1点あたり0.1秒で測定できる

測定点数 100点/台、1000点まで可能 測定点数 100点/6、1000点は、118 測定範囲 0~±60,000×10 00すみ 適用ゲージ 1200、1、2、4枚ケージ法

MELCOM70 HITAC 1011 

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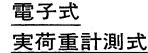
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榧

四

# 電気・油圧サーボシステム・自記計測のマルイ 創業50年

# 新しい万能材料試験機





計測

電子式万能試験機 MIE-734型

荷 特殊ロードセルは引張強度の另以下で使用 測指示は自動平衡装置利用 重負荷は多連式ポンプにて行う

※※※※的 直接計測して、 負荷荷重の検出は特殊型 『要素を省きました。 温度変化除く特殊電気回路 口 K セ ル

機構と負 従来の間接的計測の不可抗 、荷機構の分離

力

9

破断ショ

ック影響受け

な た 63

自記自動化が容易になっ

計測値 \* 油 詳細ご一報下さい。 圧系統 は関係・ は負 荷す あ りません る だけ

電子式?

# 生みだします。 くの利点を

①正確な計測 計測

**自記自動化のトップをめざす** 

指針の

ードセルは、

D·T·Fを利用

精

度 0.1

μ

すぐ参上します。

完全な電子式機構

特殊ロードセル方式のための

駆使しています 衡方式。このように計測はすべて電子回路を

動きはタコゼネレーターによる自動平

# ₩₫離圓井製作所

띪 目

非破壊試験機 土 質 試験機 アスファルト試験機 温調試験機 コンクリート試験機 水理試験装置 セメント試験機 材料試験機



⑦「0」調容易になっ

た

8 応答性早く

05秒以内

⑤再現性いちじるし

④感度上昇 ③操作簡単 ②故障発生減

⑥負荷中レ

ンジ切換えできる

少

信頼を旨とす

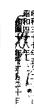
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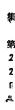
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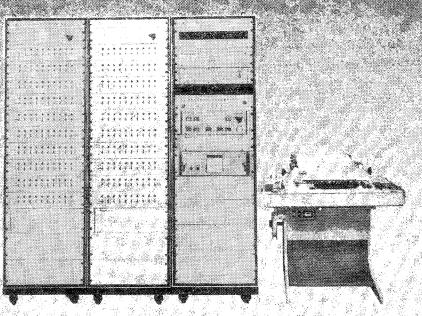
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# 高速ディジタルひずみ測 ON-LINE SE

力を自動的に切りかえて増幅し、 -D 変換するとともにオンライ ンでミニコンにインブットして解 析処理し、その出力を入出力タイ ライタにて印字作表する装置で

- ステムが選択できる
- ●中央の大型コンピュータと連結。

## 超電力の影響がない

●1点あたり0.1秒で測定できる

100点/台:1000点まで可能 測定範囲 0-±60,000×10 0 ずみ

適用ゲージ 1200、1.2、4枚ケージ法 MELGOM70 HITAC-1011

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榧