# STUDY ON NUMERICAL ANALYSIS METHODS OF SECONDARY FLOW PROFILE IN A CURVED OPEN CHANNEL

広島大学	学生会員	O Fikry Purwa Lugina
広島大学	正会員	Tatsuhiko Uchida
広島大学	正会員	Yoshihisa Kawahara

## INTRODUCTION

Understanding flow characteristics in an open channel is crucial to assess velocity distribution and sediment transport patterns for bank protection. Numerical methods have been widely used in modeling river hydrodynamics because of their cost effectiveness over experiments and field measurements. They are effective in clarifying the complex phenomena of flow structures and sediment transport after validation with reliable experimental datasets.

Two-dimensional (2D) numerical models have been applied to flows and temporal variations model in bed topographies during floods. However, 2D models present limitations when defining complex phenomena, such as three-dimensional (3D) flows. Lane et al. (1999) compared the capabilities of 2D and 3D model approaches in calculating the flow process and sediment transport; their results showed that the 3D model demonstrated a higher predictive ability. Researchers have studied 3D models (Shukla and Shiono (2008); Morvan, et. al. (2002); Jing et. al. (2008)) and reported that they demonstrated good ability in simulating flow structures in meandering channels. However, the applications of 3D models are still limited to small-scale phenomena, such as local scouring in experimental channels because of their long computational time, large memory requirements, and numerous computational tasks.

A number of depth-integrated models have been proposed to solve this problem. Uchida and Fukuoka (2016) developed a depth-integrated model, known as bottom velocity computation (BVC). The BVC method is an integrated multiscale simulation of flows and bed variations in rivers, which can evaluate vertical distributions of horizontal and bottom velocities by introducing depthaveraged horizontal vorticity and horizontal momentum equations on a water surface to shallow water equations. The BVC method with shallow water assumption is known as simplified bottom velocity computation (SBVC). The objective of this study is to analyze the ability of the BVC method in calculating flow structures in a curved open channel by comparing it with experimental data.

### METHODS

In this study, two numerical calculations were compared with the experiment by De Vriend (1979), as well as the 2D and SBVC models.. The BVC method was developed based on Eq. (1), which was derived by depth-integrating the horizontal vorticity with the shallow water assumption.

$$u_{bi} = u_{si} - \varepsilon_{ij}\Omega_j h \tag{1}$$

where  $u_{bi}$ : bottom velocity,  $u_{si}$ : water surface velocity,  $\Omega_j$ : depth-averaged vorticity, h: water depth. The bottom velocity was evaluated by the water surface velocity and depth-averaged vorticity. To evaluate the bottom velocity shown in Eq. (1), the governing equations of the BVC method were composed of the depth-integrated horizontal vorticity (Eq. (2)) and water surface velocity (Eq. (3)), in addition to the depth-integrated continuity equation (Eq. (4)) and depth-integrated horizontal momentum equation (Eq. (5)).

$$\frac{\partial \Omega_i h}{\partial t} = ER_{\sigma i} + P_{\omega i} + \frac{\partial h D_{\omega i j}}{\partial x_j}$$
(2)

where  $\Omega_i$  is the depth-averaged horizontal vorticity in the i direction,  $ER_{\sigma i}$  the rotation term of the vertical vorticity,  $P_{\omega i}$  the production term of vorticity from the bottom vortex layer, and  $D_{\omega ij}$  the horizontal vorticity flux due to convection, rotation, dispersion, and turbulence diffusion.

$$\frac{\partial u_{si}}{\partial t} + u_{sj} \frac{\partial u_{si}}{\partial x_i} = -g \frac{\partial z_s}{\partial x_i} + P_{si}$$
(3)

where g denotes gravity, and  $P_{si}$  the production term due to the shear stress acting on the thin water surface layer  $\delta z_s$ .

$$\frac{\partial h}{\partial t} + \frac{\partial U_i h}{\partial x_i} = 0 \tag{4}$$

$$\frac{\partial U_i h}{\partial t} + \frac{\partial U_i U_j h}{\partial x_j} = -gh \frac{\partial z_s}{\partial x_i} - \frac{\tau_{bi}}{\rho} + \frac{\partial h T_{ij}}{\rho \partial x_j}$$
(5)

where  $U_i$  is the depth-averaged horizontal velocity in the *i* direction,  $\tau_{bi}$  the bed shear stress, and  $T_{ij}$  the horizontal shear stress due to turbulence and vertical velocity distribution. The vertical distributions of the horizontal velocities are expressed by the cubic function (Eq. (6)) using the depth-averaged velocity  $U_i$ , velocity differences  $\delta u_i$ ,  $\Delta u_{ij}$ , and dimensionless depth  $\eta$ .

$$u_{i} = \Delta u_{i} \left( 12\eta^{3} - 12\eta^{2} + 1 \right) + \delta u_{i} \left( -4\eta^{3} + 3\eta^{2} \right) + U_{i} \quad (6)$$

where,  $\Delta u_i: u_{si} - U_i$ ,  $\delta u_i: u_{si} - u_{bi}$ ,  $\eta: (z_s - z_b)/h$ .

### **RESULTS AND DISCUSSION**

Fig. 1 shows a comparison of the depth-averaged velocity along the streamwise direction. The behavior of secondary flows in a curved channel has been discussed by de Vriend (1979); before entering the curved part, the velocity exhibited a uniform pattern. Once the velocity entered the curved part, the velocity near the inner bank decreased gradually, whereas the velocity near the outer bank

Keywords: Open channel, depth-integrated model, curved channel

Contact address: 1-4-1 Kagamiyama, Higashi-Hiroshima City, Hiroshima, 739-8527, Japan, Tel: 080-4339-5924

increased. After leaving the curved part, the outer bank became dominant owing to the large intensity of the secondary flow, which was the transverse convection of momentum transfer.

The comparison of depth-averaged velocity distributions along streamwise direction are shown in Fig. 1. Fig. 1(a) shows a comparison between the De Vriend (1979) and 2D models. The depth-averaged velocity pattern became uniform after leaving the curved part. This shows that apart from being unable to describe complex phenomena, the 2D model cannot produce the secondary flow effect in a depth-averaged velocity distribution. Fig. 2(b) shows the comparison between De Vriend (1979) and SBVC, the model shows a depth-averaged velocity distribution has similar pattern to the experimental one.

Fig. 2 shows a comparison of secondary flow structures located right at the curved part (cross section number 12). The SBVC method could reproduce secondary flow structures while 2D method could not; the major secondary flow moved outward from the wall (moved from the inner wall to the outer wall). At the surface, the flow moved to the outer wall; at the bottom, the flow moved to the inner wall. However, the SBVC model could not reproduce velocity structures at the upper-outer and bottom-outer walls. The experimental data indicated that the flow moved anticlockwise. In the SBVC model, the flow moved outward, which was one of the limitations of the SBVC model.

### CONCLUSION

The proposed model demonstrated satisfactory performance compared with the experimental data. Both the 2D and SBVC models produced a depth-averaged velocity distribution. Meanwhile, only the SBVC model could describe secondary flow structures.

#### REFERENCES

- De Vriend, D. J. (1979). Flow measurements in a Curved Rectangular Channel, Internal Report, No. 9-79.
- Jing, H., Guo, Y., Li, C., and Zhang J. (2009). Threedimensional numerical simulation of compound meandering open channel flow by the Reynolds stress model. *International Journal for Numerical Methods in Fluids*, 59:927-943.
- Lane, S. N., Bradbrook, K. F., Richards, K. S., Biron, P. A., and Roy, A. G. (1999). The application of computational fluid dynamics to natural river channels: three-dimensional versus two-dimensional approaches. *Geomorphology*, 29:1-20.
- Morvan, H., Pender, G., Wright, N.G., and Ervine, D. A. (2002). Three-dimensional hydrodynamics of meandering compound channels. *Journal of Hydraulic Engineering*, 128(7):674-782.
- Shukla, D. R. and Shiono, K. (2008). CFD modelling of meandering channel during floods. Proceedings of the Institution of Civil Engineers Water Management, 161(1):1-12.
- Uchida, T. and Fukuoka, S. (2016). Nonhydrostatic uasi-3D model coupled with dynamic rough wall law for

simulating flow over a rough bed with submerged boulders. *Journal of Hydraulic Engineering*, 142(11):04016054.



Figure 1. Depth-averaged velocity distribution along the streamwise direction. (a) De Vriend (1979) and 2D, (b) De Vriend (1979) and SBVC



Figure 2. Comparison of secondary flow structures at crosssection 12