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Modification of the Terzaghi's theory for the tunnel excavation in an inclined-layers ground

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Introduction : Experimental studies on the tunnel excavation in an inclined-layers mass have been carried out. As described in the previous research¹⁾, mechanical behaviors drawn from modeled inclined-layers ground are obviously influenced by the angle of inclined-layers. On the other hand, the theory of Terzaghi's earth pressure is selected to verify the experimental results. Original Terzaghi's theory²⁾ is, however, based on the sandy ground, so that its extension would be demanded for the inclined-layers ground. In this paper, modification to extend the original Terzaghi's theory is attempted and parametric study is performed using some kinds of parameters such as the angle of ground surface and sliding surfaces, overburden, width of tunnel and coefficient of lateral earth pressure.

For the Extended Model : In generalizing for the extended model, it was assumed that ground surface and sliding surfaces are inclined as shown in Fig. 1. Lines of sliding surfaces ac and bd having the angle θ_1 are the sliding surfaces and the angle of ground surface is θ_2 . H is defined as the average height from the yielding strip(Tunneling Part) to the ground surface that presumed from the central line of the inclined section ac and bd .

Fig. 2 shows a section through the space between two inclined sliding surfaces. Such as the original Terzaghi's model, the sum of the vertical components, which act on the small slice, must be equal to zero and this condition can be expressed by Eq.1,

$$(\bar{\sigma}_v + d\bar{\sigma}_v)D - \bar{\sigma}_v D + 2\tau dz - \gamma Dz = 0 \quad (1)$$

where γ is unit weight of soil, D is width of strip, σ_v is vertical stress, and σ_h is horizontal stress.

In the case of the inclined section, the shearing resistance (τ) of the earth should be determined by the following Eq. 2, and the normal stress (σ_n) that acts on the sliding surfaces with an angle θ_2 can be described as Eq. 3,

$$\tau = c + \bar{\sigma}_n \cdot \tan \phi \quad (2)$$

$$\bar{\sigma}_n = \frac{\bar{\sigma}_v + \bar{\sigma}_h}{2} + \frac{\bar{\sigma}_v - \bar{\sigma}_h}{2} \cos 2\theta_1 \quad (3)$$

where ϕ is internal friction angle.

By substituting in Eq. 1 into Eq. 2 and 3, the following equation can be obtained,

$$\frac{d\bar{\sigma}_v}{dz} = \frac{\gamma D - 2c}{D} \cdot \left\{ \left[(1 + K_0) + (1 - K_0) \cos 2\theta_1 \right] \bar{\sigma}_v \cdot \frac{\tan \phi}{D} \right\} \quad (4)$$

where $K_0 (= \bar{\sigma}_h / \bar{\sigma}_v)$ is coefficient of lateral earth pressure.

Solving Eq. 4 with boundary condition, $\bar{\sigma}_v = 0$ for $z = 0$, $\bar{\sigma}_v$ can be expressed as follow;

$$\bar{\sigma}_v = \frac{\gamma D - 2c}{\left\{ (1 + K_0) + (1 - K_0) \cos 2\theta_1 \right\} \tan \phi} \cdot \left(1 - \exp \left[- \left\{ (1 + K_0) + (1 - K_0) \cos 2\theta_1 \right\} \tan \phi \cdot \frac{H}{D} \left(1 - \frac{\tan \theta_2}{\tan \theta_1} \right) \right] \right) \quad (5)$$

Key words ; Terzaghi's theory, Modification, Inclined-layers ground, Tunneling, Parametric study

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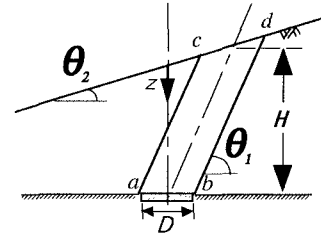


Fig. 1. Failure caused by inclined surfaces

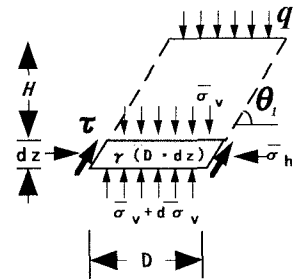


Fig. 2. Diagram illustrating extended model

One of the most crucial constraints to satisfy Eq. 5 must be confirmed whether or not the existence of the value H . Namely θ_1 should be greater than θ_2 to generate the value H . To verify Eq. 5 for generalization, by substituting successively the value $c = 0$, $\theta_1 = 90^\circ$ and $\theta_2 = 0^\circ$, it can be obtained the same equation of the original Terzaghi's equation.

Assessments of the Extended Model: Assessments are presented in terms of 5 parameters including three dimensionless factor, such as $\bar{\sigma}_v/\gamma D$ (normalized vertical load factor), H/D (normalized overburden factor), K_0 , θ_1 and θ_2 . Fig. 3, 4 and 5 give the normalized vertical load factor as a function of the normalized overburden factor for $\phi = 30^\circ$. Fig. 3 illustrates the effect of varying θ_2 on the acting load of the yielding strip for a fixed value of $\theta_1 = 60^\circ$. Fig. 4 illustrates the effect of varying θ_1 for a fixed value of $\theta_2 = 30^\circ$. And, Fig. 5 shows the effect of K_0 for a fixed value of $\theta_1 = 60^\circ$, $\theta_2 = 30^\circ$, respectively. Also in Fig. 3 and 4, K_0 is fixed at 0.5. From these figures, following results can be drawn.

1. From Fig. 3, the acting pressure on the horizontal strip decreases as θ_2 increasing. In the case of the normalized overburden factor, $H/D = 2$, the value of $\bar{\sigma}_v/\gamma D$ decreases from 1.05 to 0.62 as θ_2 varies from 0° (having horizontal surface) to 45° . These results owing to the variation of the angle of ground surface, however, cannot confirmed at the high value of the normalized overburden factor.
2. Fig. 4 shows that θ_1 has a significant effect on the load acting on the yielding strip. As the angle of the sliding surfaces decreases, the normalized vertical load factor decreases. And in all cases, the normalized vertical load factors become constant as H/D increasing.
3. It can be easily understand from Fig. 5 that, in the lower range of the normalized overburden factor 5, the normalized vertical load factor decreases rapidly. For a given normalized overburden factor 5, $\bar{\sigma}_v/\gamma D$ decreases from 1.27 to 0.62 as K_0 varies from 0.50 to 1.50. It should be also emphasized that the normalized vertical load factors become constant with increasing H/D .

Conclusion : Extended equation, based on the original Terzaghi's earth pressure is solved for the inclined-layers ground. It is concluded that the normalized vertical load factor acting on yielding strip is influenced by both the angle of sliding surfaces and ground surface.

References

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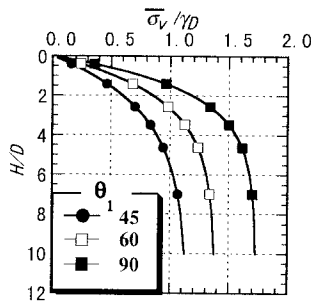


Fig. 4. Effect of the angle of sliding surfaces on normalized vertical load factor

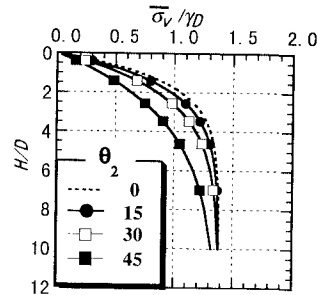


Fig. 3. Effect of the angle of ground surface on normalized vertical load factor

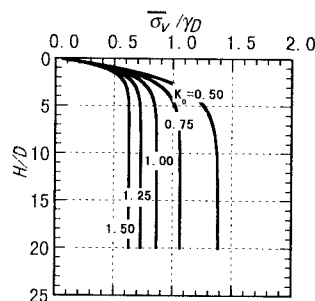


Fig. 5. Effect of the coefficient of lateral earth pressure on normalized vertical load factor