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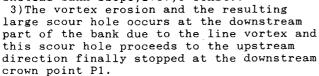
BANK EROSION DUE TO OVERFLOW

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- 1. Introduction: There are two cases on the bank erosion, i.e., the overflow erosion and the seepage erosion. Here we consider the overflow erosion where the velocity of the overflow water is higher enough to pass ahead the seepage velocity. The experimental results of the bank erosion were observed and the theoretical analysis was performed considering the momentum eq. and the stress-strain relationships for the grain-inertia regimes.
- 2. Experimental Setup: Fig. 1 shows the experimental arrangement. The bank made of sand(d=0.5mm)was put inside the recirculating rectangular channel of 0.2m wide and 3m long. A thick screen was laid at the upstream part of the channel in order to reduce the wave motion and the water was flowing from the upstream part. The video camera and the strong light were used to estimate the erosion rate.
- 3.Experimental Observations: Fig.2 and 3 shows the typical pattern of the erosion and the erosion rate respectively, and the following observations were obtained



- 1) The debris flow begins from the starting point of the granular jump and the immature debris flow occurs in front of that point.
- 2) The sheet erosion occurs in the area of the immature debris flow due to the shear stress and its slope was parallel to the initial bank slope, i.e., 0=const..



4) The bed sediments moves to the lowest part of the scour hole and suspended into the water at the right angle of the bed surface.

4. Theoretical Analysis: Consider the 2-dim. sheet flow where the erosion occurs parallel to the initial bank slope at the upper part of the downstream bank slope. In this case we assume the uniform flow and there is no net flux of sands across the surface at y=0.

Continuity eq. of the mixture

$$\frac{\partial f}{\partial t} + \frac{\partial (fu)}{\partial x} = 0$$
 $f = f(N + f(1-N))$; mixture density

Momentum eq. of the mixture

$$\frac{\partial (fu)}{\partial t} = \frac{\partial T_{xy}}{\partial y} + \int_{0}^{g} \sin \theta$$

$$0 = \frac{\partial T_{yy}}{\partial y} + \int_{0}^{g} \sin \theta$$

where, Txy; granular shear stress

Tm; granular normal stress u; mixture velocity f; fluid density f; granular density

N; granular volume fraction Until the erosion proceeds to the upstream crown point P2 of Fig.2, the crown height does not

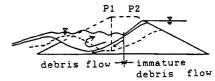


Fig. 2 Erosion pattern

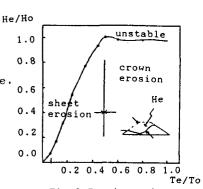


Fig.3 Erosion rate

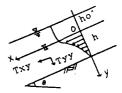


Fig. 4 Two-dim. sheet flow

decrease and the steady flow exists. Letting $\int N \ dy \cong (N_0 + l N_0) \ y = l_1 \ y$, and using the boundary condition $T_{zy} = -\beta_r \ gh_0 \ sin\theta$ at y=0, following two eqs. can be obtained. $-T_{xy} = \Delta g g l_1 y \sin \theta + g g (h_0 + y) \sin \theta$ $-T_{yy} = \Delta$ gl₁ y cose For the rapid shear flows, the stresses are, $T_{xy} = -\mu_{l} \left(\frac{2u}{2y} \right)^{2} - f_{l}gh_{o}\sin\theta \left(\frac{2u}{2y} \right)^{2} - g_{l}gh_{o}\sin\theta \left($ Na; volume fraction at the densest packing No; volume fraction at y=0 ϕ_{p} ; dynamic angle of internal friction Using the above four eqs. and the boundary condition N=Nb at y=h, the following approximate non-dim. distribution of volume fraction can be obtained. $\frac{N - N_o}{N_b - N_o} = \left(\frac{y}{h}\right)^3$ (5) where h; depth of shear layer N. -N. Fig. 5 shows eq.(5). In order to determine h, we use the dynamic Coulomb criterion, $T_{xy} = T_{yy} \tan \phi_{y}$. From (1)&(2), - 3; tan 8 The average volume fraction N is, $\frac{1}{1}\int_{0}^{h} \frac{ds}{s} ds = \frac{1}{2}\int_{0}^{h} \frac{ds}{s}$ N= 15" Ndy = 3No +Nb For the determination of the velocity profile of the shear layer, from (1)&(3), $\sqrt{3}$ $y = (H_h)$, $K_a = \frac{N_b - N_o}{N_o - N_o}$ $V = \frac{1}{3} K_{w}^{1/2} h^{\frac{4}{3}} \sum_{j=0}^{4} \frac{uC_{j} (-K_{3})^{j}}{2j+1} [1-\widetilde{\gamma}^{(2j+1)}]$ Using the boundary condition u=u at y=0,

Ho=3 Ku / 3/2 4 xC; (-K3) So, the non-dim. velocity profiles are, 1 = \$ B; (1-3(1))]/ B; $Bj = {}_{\mu}C_{3} (-K_{3})/(2j+1)$ Fig.6 shows eq. (8). The average velocity ū is, ū=1/n·5/q(y)dy

= 3 Kuh \$ [B; (1-3)+5/2)] (9)

The granular discharge Q_{s} is, $Q_{s} = S_{s} \int_{0}^{N} \mathcal{N}(y) u(y) dy = \frac{2}{3} K_{s} h^{-1/2} S_{s} \left[E_{s} \left(\vec{N} - \frac{N_{b} - N_{c}}{3j + N_{c}} - \frac{N_{b}}{3j + N_{c}} \right) \right] (10)$ The fluid discharge Q_{f} is,

 $Q = \frac{1}{2} \frac{1}{2} \frac{1}{2} \left(\frac{1}{2} - \frac{1}{2} \frac{1}{2} \frac{1}{2} - \frac{1}{2} \frac$ differences between the experimental and the theoretical values are not so large.

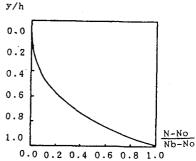


Fig. 5 Non-dim. distribution of volume fraction

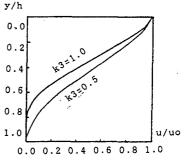


Fig. 6 Non-dim. velocity profile

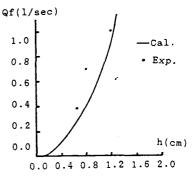


Fig.7 Relationship beween Qf and h

- 5. Conclusion; In this paper the theoretical analysis for the sheet erosion in case of the uniform flow was proposed. The theory for the the unsteady flow and the comparison between the experimental and theoretical results will be discussed later.
 - 6.References;
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- (3) Hanes D.M., and Inman D.L. (1985), Observations of rapidly flowing granular-fluid materials, J. Fluid Mech..