

## ON CASTIGLIANO'S THEOREM.

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If  $n$  forces or moments or both  $P_r$ ,  $r = 1, 2, \dots, n$ , act on a body with the corresponding displacements or rotations  $\delta_r$ , and if they are supposed to have attained their ultimate magnitudes by increasing uniformly from the initial magnitude zero, the work done on the body is

$$W = \frac{1}{2} \sum_{r=1}^n P_r \delta_r \cos(P_r, \delta_r).$$

If one of  $P$ 's, say  $P_s$ , is supposed to be  $P_s + dP_s$  instead of  $P_s$ , we have

$$\frac{\partial W}{\partial P_s} = \frac{1}{2} \sum_{r=1}^n \left( \frac{\partial P_r}{\partial P_s} \delta_r + \frac{\partial \delta_r}{\partial P_s} P_r \right) \cos(P_r, \delta_r).$$

If, on the other hand, at the loaded state of the body, we increase  $P_s$  uniformly by  $dP_s$ , we have

$$\begin{aligned} \frac{\partial W}{\partial P_s} &= \sum_{r=1}^n \left( P_r + \frac{1}{2} \frac{\partial P_r}{\partial P_s} dP_s \right) \frac{\partial \delta_r}{\partial P_s} \cos(P_r, \delta_r) \\ &= \sum_{r=1}^n \frac{\partial \delta_r}{\partial P_s} P_r \cos(P_r, \delta_r), \end{aligned}$$

neglecting the infinitesimals of first order against the finite magnitudes.

Comparing these two results, we have

$$\frac{\partial W}{\partial P_s} = \sum_{r=1}^n \frac{\partial P_r}{\partial P_s} \delta_r \cos(P_r, \delta_r).$$

In an elastic solid  $\tau$  of density  $\rho$  referred to three rectangular axes  $x$ ,  $y$ ,  $z$ , subjected to body forces  $(K_x, K_y, K_z)$  per unit mass, and with the surface  $\sigma$  subjected to surface tractions  $(T_x, T_y, T_z)$  per unit area, if  $(X_x, X_y, X_z)$ ,  $(Y_x, Y_y, Y_z)$ ,  $(Z_x, Z_y, Z_z)$  are the stresses on the faces  $dydz$ ,  $dzdx$ ,  $dx dy$  of a rectangular parallelepiped with its edges  $dx$ ,  $dy$ ,  $dz$ , parallel to the coordinate axes and with its angular point nearest to the coordinate origin at a point  $(x, y, z)$  in the interior of the body, and if  $(u_x, u_y, u_z)$  are the displacements caused by the external forces, and  $(\varepsilon_x, \varepsilon_y, \varepsilon_z)$ ,  $(\sigma_x, \sigma_y, \sigma_z)$  are the rate of elongations and shears in the interior of the body, we have, denoting the time by  $t$ ,

$$W = \int \rho \left[ \left( K_x - \frac{\partial^2 u_x}{\partial t^2} \right) u_x + \left( K_y - \frac{\partial^2 u_y}{\partial t^2} \right) u_y + \left( K_z - \frac{\partial^2 u_z}{\partial t^2} \right) u_z \right] d\tau$$

$$+ \int (T_x u_x + T_y u_y + T_z u_z) do,$$

$$U = \iiint (X_x \varepsilon_x + Y_y \varepsilon_y + Z_z \varepsilon_z + Y_x \sigma_x + Z_x \sigma_y + X_y \sigma_z) dx dy dz,$$

$U$  being the internal work done by the stresses. Making use of the equations

$$\begin{aligned} \frac{\partial X_x}{\partial x} + \frac{\partial Y_x}{\partial y} + \frac{\partial Z_x}{\partial z} + \rho \left( K_x - \frac{\partial^2 u_x}{\partial t^2} \right) &= 0 \\ \frac{\partial X_y}{\partial x} + \frac{\partial Y_y}{\partial y} + \frac{\partial Z_y}{\partial z} + \rho \left( K_y - \frac{\partial^2 u_y}{\partial t^2} \right) &= 0 \\ \frac{\partial X_z}{\partial x} + \frac{\partial Y_z}{\partial y} + \frac{\partial Z_z}{\partial z} + \rho \left( K_z - \frac{\partial^2 u_z}{\partial t^2} \right) &= 0 \\ Y_x = Z_y, \quad Z_x = X_y, \quad X_y = Y_z, \\ lX_x + mY_x + nZ_x = T_x \\ lX_y + mY_y + nZ_y = T_y \\ lX_z + mY_z + nZ_z = T_z \end{aligned}$$

in which  $l, m, n$  are the direction cosines of the outward normal on  $do$ , and the tensions are taken positive, we shall have, by Gauss' integral theorem,

$$W = U.$$

Thus we arrive at the very important result

$$\begin{aligned} \frac{\partial W}{\partial P_r} &= \sum_{r=1}^n \frac{\partial P_r}{\partial P_r} \delta_r \cos(P_r, \delta_r) \\ &= \frac{\partial U}{\partial P_r} = \frac{\partial}{\partial P_r} \iiint (X_x \varepsilon_x + Y_y \varepsilon_y + Z_z \varepsilon_z + Y_x \sigma_x + Z_x \sigma_y + X_y \sigma_z) dx dy dz. \end{aligned}$$

The current practice is to suppose  $P_r$  to be independent of  $P_s$  when  $r \neq s$ ; but this is not necessarily the case.

Last formula also applies to statically determinate as well as indeterminate elastic systems.

If there are  $\mu$  equations

$$F_a(P_1, P_2, \dots, P_n) = 0, \quad a = 1, 2, \dots, \mu,$$

and if we take  $m$  of  $P$ 's,  $m + \mu = n$ , as independent variables, we shall have

$$P_{i\alpha} = f_{i\alpha}(P_{s1}, P_{s2}, \dots, P_{s,m}), \quad \alpha = 1, 2, \dots, \mu,$$

so that

$$\frac{\partial W}{\partial P_{s\beta}} = \sum_{\alpha=1}^{\mu} \frac{\partial f_{i\alpha}}{\partial P_{s\beta}} \delta_{i\alpha} \cos(P_{i\alpha}, \delta_{i\alpha})$$

$$= \frac{\partial U}{\partial P_{\beta}^3} = \frac{\partial}{\partial P_{\beta}^3} \iiint (X_x \varepsilon_x + Y_y \varepsilon_y + Z_z \varepsilon_z + Y_x \sigma_x + Z_x \sigma_y + X_y \sigma_z) dx dy dz,$$

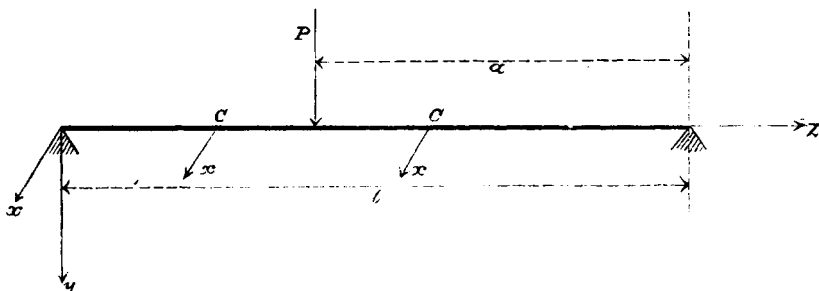
$$\beta = 1, 2, \dots, m,$$

and these  $m$  equations, together with  $\mu$  equations

$$F_{\alpha} = 0$$

may serve to determine all  $P$ 's when  $\delta$ 's are given.

Ex. 1. To find the deflection  $\delta$  at the loaded point of a simple beam of span  $l$  subjected to a single load  $P$ .



Here, if  $I$  is the moment of inertia of a normal cross section of the beam about  $x$  axis, and  $E$  the Young's modulus we easily see that

$$U = \frac{1}{2EI} \int_0^l M^2 dz$$

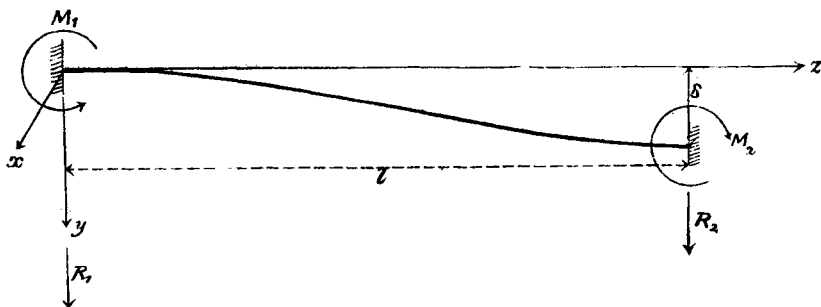
neglecting the work done by shears and denoting by  $M$  the bending moment about  $Cx$ . Thus

$$U = \frac{1}{2EI} \left[ \int_0^{l-a} \left( \frac{Pa}{l} z \right)^2 dz + \int_{l-a}^l \left\{ \frac{P(l-a)}{l} (l-z) \right\}^2 dz \right]$$

$$= \frac{P^2}{6EI} a^2 (l-a)^2.$$

$$\delta = \frac{\partial U}{\partial P} = \frac{P}{3EI} a^2 (l-a)^2.$$

Ex. 2. To find the moments and reactions of a beam of span  $l$ , whose one end is absolutely fixed and whose other end is fixed against the rotation and is displaced by  $\delta$  normal to the beam.



Giving to  $I$  and  $E$  the same significations as before, and neglecting the work done by shears we have

$$U = \frac{1}{2EI} \int_0^l (M_1 + R_1 z)^2 dz$$

$$= \frac{1}{2EI} (M_1^2 l + M_1 R_1 l^2 + \frac{R_1^2}{3} l^3),$$

and by the principle of Statics,

$$M_1 - M_2 + R_1 l = 0$$

$$R_1 + R_2 = 0.$$

If we take  $M_1$  and  $R_1$  as independent variables, we have

$$\frac{\partial U}{\partial R_1} = \frac{1}{2EI} (M_1 l^2 + \frac{2R_1}{3} l^3) = \delta$$

$$\frac{\partial U}{\partial M_1} = \frac{1}{2EI} (2M_1 l + R_1 l^2) = 0,$$

whence it follows that

$$R_1 = -R_2 = -\frac{12EI\delta}{l^3}$$

$$M_1 = -M_2 + \frac{6EI\delta}{l^2}.$$

If we take  $M_1$  and  $M_2$  as independent variables, it will be convenient, although not necessary, to put  $U$  in the form

$$U = \frac{l}{6EI} (M_1^2 + M_1 M_2 + M_2^2).$$

Thus we have

$$\frac{\partial U}{\partial M_1} = \frac{l}{6EI} (2M_1 + M_2) = \frac{\delta}{l}$$

$$\frac{\partial U}{\partial M_2} = \frac{l}{6EI} (M_1 + 2M_2) = -\frac{\delta}{l},$$

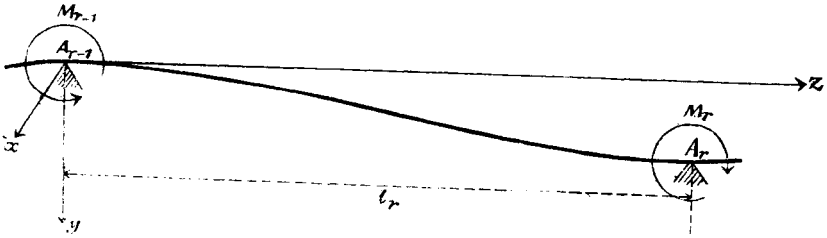
whence it follows that

$$M_1 = -M_2 = \frac{6EI\delta}{l^2}$$

$$R_1 = -R_2 = -\frac{12EI\delta}{l^3},$$

the same results as before.

Ex. 3. To find the theorem of three moments.



Let  $M$  denote the bending moment for  $r$ th span considered as a simple beam, then the reactions at  $A_{r-1}$  and  $A_r$  are

$$-\frac{M_{r-1} + M_r}{l_r} \quad \text{and} \quad \frac{M_{r-1} - M_r}{l_r}$$

respectively. Hence giving to  $I$  and  $E$  the same significations as before, and neglecting the work done by shears, we have

$$\begin{aligned} U_r &= \frac{1}{2EI} \int_0^{l_r} \left( M_{r-1} + \frac{-M_{r-1} + M_r}{l_r} z + M \right) z \, dz \\ &= \frac{l_r}{6EI} (M_{r-1}^2 + M_{r-1}M_r + M_r^2) + \frac{M_{r-1}}{EI} \int_0^{l_r} M \, dz \\ &\quad - \frac{M_{r-1} - M_r}{EI l_r} \int_0^{l_r} Mz \, dz + \frac{1}{2EI} \int_0^{l_r} M^2 \, dz \end{aligned}$$

Thus we obtain

$$\frac{\partial U_r}{\partial M_r} = \frac{l_r}{6EI} (M_{r-1} + 2M_r) + \frac{1}{EI l_r} \int_0^{l_r} Mz \, dz = -\varphi_r + \frac{\delta_{r-1} - \delta_r}{l_r},$$

$\varphi_r$  being the rotation at  $A_r$ .

Similarly for  $(r+1)$ th span, we have

$$U_{r+1} = \frac{l_{r+1}}{6EI} (M_r^2 + M_r M_{r+1} + M_{r+1}^2) + \frac{M_r}{EI} \int_0^{l_{r+1}} M \, dz \\ - \frac{M_r - M_{r+1}}{EI l_{r+1}} \int_0^{l_{r-1}} Mz \, dz + \frac{1}{2EI} \int_0^{l_{r+1}} M^2 \, dz,$$

so that

$$\frac{\partial U_{r+1}}{\partial M_r} = \frac{l_{r+1}}{6EI} (2M_r + M_{r+1}) + \frac{1}{EI l_{r+1}} \int_0^{l_{r+1}} M (l_{r+1} - z) \, dz \\ = \varphi_r - \frac{\delta_r - \delta_{r+1}}{l_{r+1}}.$$

Adding these partial derivatives of  $U_r$  and  $U_{r+1}$  with respect to  $M_r$ , we have, after multiplying the result by  $6EI$  and transposing the integral terms,

$$l_r M_{r-1} + 2(l_r + l_{r+1}) M_r + l_{r+1} M_{r+1} \\ = -\frac{6}{l_r} \int_0^{l_r} Mz \, dz - \frac{6}{l_{r+1}} \int_0^{l_{r+1}} M (l_{r+1} - z) \, dz \\ - 6EI \left( \frac{\delta_r - \delta_{r-1}}{l_r} + \frac{\delta_r - \delta_{r+1}}{l_{r+1}} \right),$$

a well-known theorem of three moments.

The case when  $I$  and  $E$  are not constant may be treated in an exactly similar manner.

It will be noticed that by finding the partial derivatives of  $U_r$  with respect to  $M_{r-1}$  and  $M_r$ , we shall have the solution of the general case of Ex. 2.